ITB 19-05 January 17, 2019

#### ADDENDUM NO. 3

#### BID NO. 19-05

#### RIPKEN STADIUM STRUCTURAL REPAIR CITY OF ABERDEEN, MARYLAND

To all holders of the specifications, the following corrections are hereby made. All other items shall remain unchanged. This addendum shall become part of the Contract Documents for the above referenced project.

Acknowled	gement d	of A	ddendum
			············

Name:	 	 
Company:		
Date:		 
Signature:		

- Section 1: Responses to Questions
- Section 2: Previous Tieback Installation Report

#### Section 1 - The following are answers to questions posed by bidders:

- **Q1:** Has a start date been determined?
- **A1:** The City estimates a start date between February 15, 2019 and March 15, 2019.
- **Q2:** Do you want control on the survey? If yes, where is the control for the survey?
- **A2:** Yes, control points should be set on the site. Offsite control is not required.
- Q3: Can the current electrical box/chain link fence be reused, or should it be replaced?
- **A3:** Yes, if the materials are salvaged in its current condition, they can be re-used.
- **Q4:** Can the sacrificial micropile be tested in tension? How close does the pile need to be to the work area?
- **A4:** The pile load test should be in compression. Test within 10 feet of the proposed work.
- **Q5:** Does the sacrificial pile need to be removed completely?
- **A5:** Cut off non-production test piles 2 feet below grade.

#### **Bid Form**

**Q6:** Is the Contractor responsible for final as-builts?

**A6:** The Contractor is responsible for final as-builts.

**Q7:** Are there any drilling or grouting methods that will not be considered?

**A7:** No.

**Q8:** Are the micropiles to be pressure grouted?

**A8:** Gravity-grouted micropiles were envisioned. Other recognized grout placement methods can be considered.

**Q9:** Are the tiebacks to be pressure grouted or gravity grouted before post-grouting?

**A9:** Gravity grouting of tiebacks prior to re-grouting was envisioned. Other recognized grout placement methods can be considered.

**Q10:** Are the tiebacks to be post-grouted? If so, is there a minimum grout pressure to be reached?

**A10:** Post-grouted tiebacks are depicted on the plans. Pressure sufficient to introduce new grout into the element is required.

**Q11:** Is the grouting performance based? Meaning if the anchor test holds after one round of grouting, does post-grouting need to be completed?

**A11:** A successful load test is required for acceptance. Contractor-requested adjustments to the initially-tested configuration are subject to re-testing and should be no additional cost to the owner.

Q12: Current design only allows for 6-inch of clearance between the inner wall of the boring and the existing retaining wall. This may not be enough clearance for the drill rig. Is it possible to adjust the boring a few more inches from the wall if necessary?

**A12:** Small adjustment is acceptable, but accommodating the relocation is likely to require minor revision to the concrete and reinforcing and should be no additional cost to the Owner.

Q13: Can we discharge processed and filtered drilling effluent to the storm drain?

A13: No.

Q14: What will the finished surface on the exposed face of the wall be?

**A14:** The wall finish will be smooth form finish with trowel finish on the top surface.

Q15: Do you have access to any data relating to the previous anchor installation?

**A15:** See attached Section 2 containing information on the previous anchor installation.

# CCGI Creative Concepts Group, Inc.

**Specialty Foundation Consultants & Contractors** 



#### **Facsimile**

Contact Name: Randy Robertson | Company Name: City of Aberdeen

Date: 6-13-05 | Fax Number: 410-273-7402 | Phone Number: 410-297-4217

Number Pages Including Cover Sheet: 5 From: Chris DeBlauw

#### Comments:

#### Hello Mr. Robertson

Attached are all of the pile installation reports and the test reports for all 9 tiebacks. One of the tiebacks, #6, was installed 7 feet deeper than the others, as we had equipment problems on that anchor and had to take it out twice. That happens to be the tieback that performed the best and it may have been conservative to add length, but we like to be sure about these things. You do have some issues related to the concrete structures and water table that I would like to discuss with you when you have a moment. If you have any questions related to this correspondence, please give me a call. I will also forward these items to Davis Bowen Friedel.

Thank you,

Chris DeBlauw Cell# 443-254-3237

CC: DDR

Phone # 410-923-9830 www.ccgicorp.com 802 Mallet Hill Lane Millersville, MD. 21108

Fax # 410-923-5780 www.ccgicorp.com

# CCGI CREATIVE CONCEPTS GROUP, INC. Piling Installation Report



802 - R Mallet Hill Lane, Millersville, MD 21108, Phone# 410-923-9830, Fax# 410-923-5780, Email - cd@ccglcorp.com

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#### CCGI CREATIVE CONCEPTS GROUP, INC. **Piling Installation Report**

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### Page 3

## CCGI CREATIVE CONCEPTS GROUP, INC.

Piling Installation Report

802 - R Mallet Hill Lane, Millersville, MD 21108, Phone# 410-923-9830, Fax# 410-923-5780, Email - cd@ccglcorp.com



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### **CCGI**

### CREATIVE CONCEPTS GROUP, INC.



			Anchor Proof	Test	
Date:	6/9/2005	Design Load (Kips):	22.4	Project:	Cal Ripken Stadium
	Jac	ck Calibration (PSI/Klp):	74		

	Applie	d Load (K	(lps)	Jack PSI	#1	#2	#3	#4	#5	#6	#7	#8	#9
+	0.25	DL	5.6	414	0.048	0.058	0.014	0.011	0.024	0.029	0.020	0.011	0.002
	0.50	DL	11.2	829	0.091	0.141	0.058	0.024	0.046	0.036	0.084	0.010	0.002
	0.75	DL	16.8	1,243	0.148	0.244	0.104	0.045	0.078	0.032	0.112	0.152	0.016
	1.00	DL	22.4	1,658	0.209	0.352	0.160	0.056	0.116	0.032	0.287	0.203	0.025
	1.20	DL	26.88	1,989	0.245	0.430	0.193	0.069	0.135	0.035	0.300	0.245	0.037
_	1.33	DL	29.792	2,205	0.260	0.534	0.219	0.078	0.148	0.068	0.336	0.267	0.055
	1.50	DL	33.6	2,486	0.292	0.611	0.252	0.090	0.160	0.085	0.348	0.309	0.080
+	0.00	DL	0	-	0.283	0.600	0.246	0.084	0.150	0.073	0.341	0.301	0.075
es:	AL = Align	ment Loa	d, DL = D	esign Loa	d	A	II measure	ments in in	nches				
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Note: During the load application of pile #2 the bearing plate moved down slightly after the second and third increments. It is my opinoin from pre-load testing that this movement may of effected the end result by as much as two tenths of an inch.